

September 30, 2020

1200 New Jersey Ave., SE Washington, D.C. 20590

In Reply Refer To: HSST-1/B-349

Mr. Samuel Summerville Thornton Tomasetti, Inc. 40 Wall Street New York, NY 10005 USA

Dear Mr. Summerville:

This letter is in response to your June 12, 2020 request for the Federal Highway Administration (FHWA) to review a roadside safety device, hardware, or system for eligibility for reimbursement under the Federal-aid highway program. This FHWA letter of eligibility is assigned FHWA control number B-349 and is valid until a subsequent letter is issued by FHWA that expressly references this device.

Decision

The following device is eligible within the length-of-need, with details provided in the form which is attached as an integral part of this letter:

• Throgs Neck Bridge Rail

Scope of this Letter

To be found eligible for Federal-aid funding, new roadside safety devices should meet the crash test and evaluation criteria contained in the American Association of State Highway and Transportation Officials'(AASHTO) Manual for Assessing Safety Hardware (MASH). However, the FHWA, the Department of Transportation, and the United States Government do not regulate the manufacture of roadside safety devices. Eligibility for reimbursement under the Federal-aid highway program does not establish approval, certification or endorsement of the device for any particular purpose or use.

This letter is not a determination by the FHWA, the Department of Transportation, or the United States Government that a vehicle crash involving the device will result in any particular outcome, nor is it a guarantee of the in-service performance of this device. Proper manufacturing, installation, and maintenance are required in order for this device to function as tested.

This finding of eligibility is limited to the crashworthiness of the system and does not cover other structural features, nor conformity with the Manual on Uniform Traffic Control Devices.

Eligibility for Reimbursement

Based solely on a review of crash test results and certifications submitted by the manufacturer, and the crash test laboratory, FHWA agrees that the device described herein meets the crash test and evaluation criteria of the AASHTO's MASH. Therefore, the device is eligible for reimbursement under the Federal-aid highway program if installed under the range of tested conditions.

Name of system: Throgs Neck Bridge Rail Type of system: Longitudinal Barrier Test Level: MASH Test Level 5 (TL5) Testing conducted by: Texas A&M Transportation Institute (TTI) Date of request: June 12, 2020

FHWA concurs with the recommendation of the accredited crash testing laboratory on the attached form.

Full Description of the Eligible Device

The device and supporting documentation, including reports of the crash tests or other testing done, videos of any crash testing, and/or drawings of the device, are described in the attached form.

Notice

This eligibility letter is issued for the subject device as tested. Modifications made to the device are not covered by this letter. Any modifications to this device should be submitted to the user (i.e., state DOT) as per their requirements.

You are expected to supply potential users with sufficient information on design, installation and maintenance requirements to ensure proper performance.

You are expected to certify to potential users that the hardware furnished has the same chemistry, mechanical properties, and geometry as that submitted for review, and that it will meet the test and evaluation criteria of AASHTO's MASH.

Issuance of this letter does not convey property rights of any sort or any exclusive privilege. This letter is based on the premise that information and reports submitted by you are accurate and correct. We reserve the right to modify or revoke this letter if: (1) there are any inaccuracies in the information submitted in support of your request for this letter, (2) the qualification testing was flawed, (3) in-service performance or other information reveals safety problems, (4) the system is significantly different from the version that was crash tested, or (5) any other information indicates that the letter was issued in error or otherwise does not reflect full and complete information about the crashworthiness of the system.

Standard Provisions

- To prevent misunderstanding by others, this letter of eligibility designated as FHWA control number B-349 shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed upon request.
- This letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented system for which the applicant is not the patent holder.
- This FHWA eligibility letter is not an expression of any Agency view, position, or determination of validity, scope, or ownership of any intellectual property rights to a specific device or design. Further, this letter does not impute any distribution or licensing rights to the requester. This FHWA eligibility letter determination is made based solely on the crash-testing information submitted by the requester. The FHWA reserves the right to review and revoke an earlier eligibility determination after receipt of subsequent information related to crash testing.

Sincerely,

Michael S. Griffith Director, Office of Safety Technologies Office of Safety

Enclosures

Version 10.0 (05/16) Page 1 of 6 Request for Federal Aid Reimbursement Eligibility of Highway Safety Hardware

	Date of Request:	June 12, 2020	New	
Name: Samuel Sum		Samuel Summerville		
tter	Company:	Thornton Tomasetti, Inc.		
Address: 40 Wall Street, New		40 Wall Street, New York, NY 1000	5	
Suł	Country:	USA		
	To:	Michael S. Griffith, Director FHWA, Office of Safety Technologies		

I request the following devices be considered eligible for reimbursement under the Federal-aid highway program.

Device & Testing Criterion - Enter from right to left starting with Test L 1 - 1

System Type	Submission Type	Device Name / Variant	Testing Criterion	Test Level
'B':Rigid/Semi-Rigid Barriers (Roadside, Median, Bridge Railings)	Physical Crash Testing Engineering Analysis	Throgs Neck Bridge Rail	AASHTO MASH	TL5

By submitting this request for review and evaluation by the Federal Highway Administration, I certify that the product(s) was (were) tested in conformity with the AASHTO Manual for Assessing Safety Hardware and that the evaluation results meet the appropriate evaluation criteria in the MASH.

Individual or Organization responsible for the product:

Contact Name:	Edmond Knightly	Same asSubmitter		
Company Name: MTA Bridges and Tunnels Same asSubm		Same asSubmitter		
Address:	4260 Throgs Neck Expressway, Bronx, NY 10465	Same asSubmitter		
Country:	USA	Same asSubmitter		
Enter below all disclosures of financial interests as required by the FHWA `Federal-Aid Reimbursement Eligibility Process for Safety Hardware Devices' document.				
Texas A&M Transportation Institute (TTI) was contracted by MTABridges and Tunnels and Thornton Tomasetti, Inc. to perform full-scale crash testing of the Throgs Neck Bridge Rail. There are no shared financial interests in the Throgs Neck Bridge Rail by TTI, or between/among MTABridges and Tunnels and Thornton Tomasetti, Inc. and TTI, other than compensation of costs involved in the actual crash tests and reports for this submission to FHWA. Thornton Tomasetti holds no financial interests in the testing of the Throgs Neck Bridge Rail. Thornton Tomasetti does not benefit from the Throgs Neck Bridge Rail manufacture or future use, and will be compensated for their work by MTABridges and Tunnels regardless of acceptance by FHWA.				
610921-01-1,2,3				

PRODUCT DESCRIPTION

Help	
_ Nev	v Hardware or

Significant Modification

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O Modification to Existing Hardware

The test installation was a bridge rail, comprised of four HSS tubular rails, with the top and bottom rail each being $5 \times 3 \times \frac{1}{2}$ inch and the two middle rails each being $6 \times 6 \times \frac{3}{2}$ inch. The top of the uppermost of the four rails was 3 ft-6 inches from the top of the asphalt overlay, with 6-inch vertical spacings between the rails. The rails were supported by 29 posts, of which, 11 were installed on a 50 ft-10 inches long orthotropic steel deck, 4 were installed on a concrete beam foundation adjacent to the steel deck on the upstream side, and 14 were installed on a concrete beam foundation adjacent to the downstream side of the steel deck. The posts were spaced at 4 ft-11¹/₄ inches on the steel deck, with some variations in spacing for the postsat either end on the concrete beam foundations. Total installation length was 136 ft 3 inches. The orthotropic steel deck was overlaid with a 2-inch thick lift of asphalt.

CRASH TESTING

Bysignature below, the Engineer affiliated with the testing laboratory, agrees in support of this submission that all of the critical and relevant crash tests for this device listed above were conducted to meet the MASH test criteria. The Engineer has determined that no other crash tests are necessary to determine the device meets the MASH criteria.

Engineer Name: Nauman Sheikh		
Engineer Signature:	Nauman Sheikh Digitally sign Date: 2020.	ed by Nauman Sheikh 06.11 13:06:43 -05'00'
Address:	1254 Avenue A, Bldg 7091, Bryan, Texas 77807	Same asSubmitter
Country:	USA	Same asSubmitter

Help

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Required Test Number	Narrative Description	Evaluation Results
	Test 5-10 involves an 1100C vehicle impacting the test article at a target impact speed of 62 mi/h and target angle of 25°. The target CIP for the right corner of the front bumper was 3.6 ft upstream of the splice between Posts 7 & 8.	
	The results of the test conducted on February 3, 2020, are found in TTI Test Report No. 610921-01. The test vehicle was traveling at aspeed of 63.4 mi/h as it made contact with the Throgs Neck Bridge Rail 3.3 ft upstream of the splice between Posts 7 & 8 and at an impact angle of 25.1°. After loss of contact with the bridge rail, the vehicle came to rest 201 ft downstream of the impact point and 16 ft toward the field side.	
	The Throgs Neck Bridge Rail contained and redirected the 1100C vehicle. The vehicle did not penetrate, underride, or override the installation. The vehicle exited within the exit box criteria defined in MASH.	
5-10 (1100C)	Maximum dynamic deflection of the rail during the test was 0.7 inch. No permanent deformation was observed. Working width was 16 inches.	PASS
	No detached elements, fragments, or other debris were present to penetrate, or to show potential for penetrating, the occupant compartment, or to present undue hazard for others in the area.	
	The 1100C vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 9° and 16°, respectively. Longitudinal OIV was 21.3 ft/s and lateral	
	OIV was 35.1 ft/s. Maximum longitudinal occupant ridedown acceleration was 4.1 g, and maximum lateral occupant ridedown acceleration was 13.3 g. Occupant risk factors were within the maximum limitsspecified in MASH.	
	Maximum exterior crush to the vehicle was 12.0 inches in the side plane in the front plane at the left front corner at bumper height. Maximum occupant compartment deformation was 2.25 inches in the right front floor pan area.	
	The Throgs Neck Bridge Rail performed acceptably for MASH test 5-10.	

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Required Test Number	Narrative Description	Evaluation Results
	Test 5-11 involves a 2270P vehicle impacting the test article at a target impact speed of 62 mi/h and target angle of 25°. The target CIP for the right corner of the front bumper was 4.3 ft upstream of the splice between Posts7 & 8.	
	The results of the test conducted on February 7, 2020, are found in TTI Test Report No. 610921-01. The test vehicle was traveling at aspeed of 63.8 mi/h as it made contact with the Throgs Neck Bridge Rail 4.2 ft upstream of the splice between Posts 7 & 8 and at an impact angle of 25.5°. After loss of contact with the bridge rail, the vehicle came to rest 206 ft downstream of the impact point and 5 ft toward the field side.	
	The Throgs Neck Bridge Rail contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. The vehicle exited within the exit box criteria defined in MASH.	
5-11 (2270P)	Maximum dynamic deflection during the test was 1.45 inches. No permanent deformation was observed. Working width was 16 inches.	PASS
	No detached elements, fragments, or other debris were present to penetrate, or to show potential for penetrating, the occupant compartment, or to present undue hazard for others in the area.	
	The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 18° and 10°, respectively. Longitudinal OIV was 24.3 ft/s and lateral	
	Maximum longitudinal occupant ridedown acceleration was 5.9 g and maximum lateral occupant ridedown acceleration was 12.0 g. Occupant risk factors were within the maximum limitsspecified in MASH.	
	Maximum exterior crush to the vehicle was 16.0 inches in the side plane at the right front corner at bumper height. Maximum occupant compartment deformation was 3.25 inch in the right side firewall area.	
	The Throgs Neck Bridge Rail performed acceptably for MASH test 5-11.	

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5-12 (36000V)	Test 5-12 involves a 36000V vehicle impacting the test article at a target impact speed of 50 mi/h and target angle of 15°. The target CIP for the right corner of the front bumper was 1.0 ft downstream of splice between Posts 7 & 8. The results of the test conducted on March 2, 2020, are found in TTITestReport No. 610921-01. The test vehicle was traveling at aspeed of 50.8 mi/h as it made contact with the Throgs Neck Bridge Rail 4.3 inches downstream of the splice between Posts 7 & 8 at an impact angle of 14.1°. After loss of contact with the bridge rail, the vehicle came to rest 288 ft downstream of the impact point and 28 ft toward the traffic lanesside. The Throgs Neck Bridge Rail contained and redirected the 36000V vehicle. The vehicle did not penetrate, underride, or override the installation. The vehicle exited within the exit box criteria defined in MASH. Maximum dynamic deflection during the test was 1.4 inches. Maximum permanent deformation was 1.25 inches. Working width was 41.3 inches. No detached elements, fragments, or other debris were present to penetrate or to show potential for penetrating the occupant compartment, or to present undue hazard for others in the area. The 36000V vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 10° and50°, respectively. Longitudinal OIV was 2.3 ft/s, and lateral OIV was 9.8 ft/s. Maximum longitudinal occupant ridedown acceleration was 9.5 g, and maximum lateral occupant ridedown acceleration was 29.4 g. Maximum exterior crush to the vehicle was 16.0 inches in the side plane at the right front corner at bumper height. No occupant compartment deformation was observed. The Throgs Neck Bridge Rail performed acceptably for MASH test 5-12.	PASS
	acceptably for MASH test 5-12.	

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5-20 (1100C)	Tests for transition is not applicable for this bridge rail barrier system	Non-Relevant Test, not conducted
5-21 (2270P)	Tests for transition is not applicable for this bridge rail barrier system	Non-Relevant Test, not conducted
5-22 (36000V)	Tests for transition is not applicable for this bridge rail barrier system	Non-Relevant Test, not conducted

Full Scale Crash Testing was done in compliance with MASH by the following accredited crash test laboratory (cite the laboratory's accreditation status as noted in the crash test reports.):

Laboratory Name:	Texas A&M Transportation Institute		
LaboratorySignature:	Digitally signed by Darrell L. Kuhn 'Date: 2020.06.11 13:41:48-05'00		
Address:	1254 Avenue A, Bldg 7091, Bryan, Texas 77807	Same asSubmitter	
Country:	USA	Same asSubmitter	
Accreditation Certificate Number and Dates of current Accreditation period :	ISO 17025-2017 Laboratory A2LA Certificate Number: 2821.01 Valid To: April 30, 2021		

Submitter Signature*: Samuel

Digitally signed by Samuel Summerville Date: 2020.07.01 14:53:30 -04'00'

Submit Form

ATTACHMENTS

Attach to this form:

- 1) Additional disclosures of related financial interest as indicated above.
- 2) A copy of the full test report, video, and a Test Data Summary Sheet for each test conducted in support of this request.
- 3) A drawing or drawings of the device(s) that conform to the Task Force-13 Drawing Specifications [Hardware Guide Drawing Standards]. For proprietary products, a single isometric line drawing is usually acceptable to illustrate the product, with detailed specifications, intended use, and contact information provided on the reverse. Additional drawings (not in TF-13 format) showing details that are relevant to understanding the dimensions and performance of the device should also be submitted to facilitate our review.

FHWA Official Business Only:

Eligibility Letter		
Number	Date	Key Words



General Information		Impact Conditions	Post-Impact Trajectory
Test Agency	Texas A&M Transportation Institute (TTI)		Stopping Distance
Test Standard Test No	MASH Test 5-10	Angle 25.1°	16 ft twd field side
TTI Test No	610921-01-1	Location/Orientation 3.3 ft upstream of	Vehicle Stability
Test Date	2020-02-03	splice btw posts 7&8	Maximum Yaw Angle 39°
Test Article		Impact Severity 59 kip-ft	Maximum Pitch Angle 16°
Туре	Longitudinal Barrier – Bridge Rail	Exit Conditions	Maximum Roll Angle 9°
Name	Throgs Neck Bridge Rail	Speed 50.8 mi/h	Vehicle Snagging No
Installation Length	136 ft-3 inches	Trajectory/Heading Angle 7.2° / 7.2°	Vehicle Pocketing No
Material or Key Elements	Four HSS square tubing, top and bottom	Occupant Risk Values	Test Article Deflections
	rails $5 \times 3 \times \frac{1}{2}$ inches and two middle rails	Longitudinal OIV 21.3 ft/s	Dynamic 0.7 inch
	6 × 6 × 3/8 inches supported by steel posts	Lateral OIV 35.1 ft/s	Permanent None
Soil Type and Condition	Steel orthotropic deck with asphalt	Longitudinal Ridedown 4.1 g	Working Width 16.0 inches
	overlay, Dry	Lateral Ridedown 13.3 g	Height of Working Width 1.75 inches
Test Vehicle	1100C	THIV 12.3 m/s	Vehicle Damage
Type/Designation	2009 Kia Rio	ASI 2.81 g	VDS 01RFQ5
Make and Model	2435 lb	Max. 0.050-s Average	CDC 01FREW4
Curb	2440 lb	Longitudinal12.2 g	Max. Exterior Deformation 12.0 inches
Test Inertial	165 lb	Lateral21.6 g	OCDI FR0100000
Dummy	2605 lb	Vertical 3.2 g	Max. Occupant Compartment
Gross Static			Deformation 2.25 inches



General Information		Impact Conditions	Post-Impact Trajectory	
Test Agency	Texas A&M Transportation Institute (TTI)	Speed 63.8 mi	i/h Stopping Distance	206 ft downstream
Test Standard Test No	MASH Test 5-11	Angle 25.5°		5 ft twd field side
TTI Test No	610921-01-2	Location/Orientation 4.2 ft u	pstream of Vehicle Stability	
Test Date	2020-02-07	splice b	otw posts 7&8 Maximum Yaw Angle	44°
Test Article		Impact Severity 128 kip	p-ft Maximum Pitch Angle	10°
Туре	Longitudinal Barrier – Bridge Rail	Exit Conditions	Maximum Roll Angle	18°
Name	Throgs Neck Bridge Rail	Speed 51.2 mi	i/h Vehicle Snagging	No
Installation Length	136 ft-3 inches	Trajectory/Heading Angle 6.7° / 1	3.7° Vehicle Pocketing	No
Material or Key Elements	Four HSS square tubing, top and bottom	Occupant Risk Values	Test Article Deflections	
-	rails $5 \times 3 \times \frac{1}{2}$ inches and two middle rails	Longitudinal OIV 24.3 ft/s	s Dynamic	1.45 inches
	$6 \times 6 \times \frac{3}{8}$ inches supported by steel posts	Lateral OIV 33.5 ft/s	s Permanent	None
Soil Type and Condition	Steel orthotropic deck with asphalt	Longitudinal Ridedown 5.9 g	Working Width	16.0 inches
	overlay, Dry	Lateral Ridedown 12.0 g	Height of Working Width	1.75 inches
Test Vehicle		THIV 12.5 m/	/s Vehicle Damage	
Type/Designation	2270P	ASI 2.54	VDS	01RFQ5
Make and Model	2014 RAM 1500 Pickup Truck	Max. 0.050-s Average	CDC	01FREW4
Curb	5008 lb	Longitudinal13.6 g	Max. Exterior Deformation	16.0 inches
Test Inertial	5064 lb	Lateral	OCDI	FR0020000
Dummy	165 lb	Vertical 3.2 g	Max. Occupant Compartment	
Gross Static	5229 lb	-	Deformation	3.25 inches

Figure 6.6. Summary of Results for MASH Test 5-11 on Throgs Neck Bridge Rail.

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TR No. 610921-01

General Information		Impact Conditions	Post-Impact Trajectory	
Test Agency	Texas A&M Transportation Institute (TTI)	Speed 50.8 mi/h	Stopping Distance	288 ft downstream
Test Standard Test No	MASH Test 5-12	Angle 14.1°		28 ft twd traffic lane
TTI Test No	610921-01-3	Location/Orientation 4.3 inches dwnstrm	Vehicle Stability	
Test Date	2020-03-02	of splice btwn 7 & 8	Maximum Yaw Angle	27°
Test Article		Impact Severity 411 kip-ft	Maximum Pitch Angle	50°
Туре	Longitudinal Barrier – Bridge Rail	Exit Conditions	Maximum Roll Angle	10°
Name	Throgs Neck Bridge Rail	Speed Not obtainable	Vehicle Snagging	No
Installation Length	136 ft-3 inches	Trajectory/Heading Angle Not obtainable	Vehicle Pocketing	No
Material or Key Elements.	Four HSS square tubing, top and bottom	Occupant Risk Values	Test Article Deflections	
	rails $5 \times 3 \times \frac{1}{2}$ inches and two middle rails	Longitudinal OIV 2.3 ft/s	Dynamic	1.44 inches
	6 × 6 × 3/8 inches supported by steel posts	Lateral OIV 9.8 ft/s	Permanent	1.25 inches
Soil Type and Condition	Steel orthotropic deck with asphalt overlay,	Longitudinal Ridedown 9.5 g	Working Width	41.3 inches
	Dry	Lateral Ridedown 29.4 g	Height of Working Width	125.1 inches
Test Vehicle	•	THIV 1.8 m/s	Vehicle Damage	
Type/Designation	36000V	ASI 1.53	VDS	NA
Make and Model	2006 International 8600 w/2002 53-ft Trailer	Max. 0.050-s Average	CDC	NA
Curb	29,270 lb	Longitudinal	Max. Exterior Deformation	16.0 inches
Test Inertial	80,230 lb	Lateral	OCDI	NA
Dummy	No dummy	Vertical 10.1 g	Max. Occupant Compartment	
Gross Static	80,230 lb	0	Deformation	None

Figure 7.6. Summary of Results for *MASH* Test 5-12 on Throgs Neck Bridge Rail.

TRIBOROUGH BRIDGE AND TUNNEL AUTHORITY

PROJECT GFM-520N, WORK ORDER #14 BARRIER CRASH TESTING FOR PROJECT TN-49

JULY 6, 2018

PREPARED BY: ThorntonTomasetti 40 WALL STREET, NEW YORK, NY 10005

SAMUEL SUMMERVILLE, P.E. PRINCIPAL - THORNTON TOMASETTI

7/26/18 DATE

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DRAWING LIST

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SHEET NO.	DWG. NO.	REV. NO.	DRAWING TITLE
			GENERAL
1	G-1	0	TITLE SHEET AND LOCATION PLAN
2	G-2	0	DRAWING LIST. SYMBOLS AND ABBREVIATIONS
3	G-3	0	GENERAL NOTES
			STRUCTURAL
4	S-1	0	ORTHOTROPIC DECK PANEL AND RAILING PLAN
5	S-2	0	FLOOR TRUSS DETAILS - 1
6	S-3	0	FLOOR TRUSS DETAILS - 2
7	S-4	0	LONGITUDINAL TRUSS DETAILS
8	S-5	0	DIAPHRAGM DETAILS - 1
9	S-6	0	DIAPHRAGM DETAILS - 2
10	SB-1	0	STEEL RAILING DETAILS - 1
11	SB-2	0	STEEL RAILING DETAILS - 2
12	SB-3	0	STEEL RAILING DETAILS - 3

SYMBOLS

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LEGENDS FOR SECTION MARKS

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A DETAIL 00-00 DRAWING NUMBER ON WHICH DETAIL IS SHOWN

> SECTION MARK 00-00 - DRAWING NUMBER ON WHICH SECTION IS SHOWN

ABT	ABOUT	NO	NUMBER
ANCH	ANCHORAGE	NOM	NOMINAL
AOBE	AS ORDERED BY ENGINEER	NS	NEAR SIDE
ASTM	AMERICAN SOCIETY OF	NTS	NOT TO SCALE
	TESTING MATERIALS	OC	ON CENTER
B IO B \ BB	BACK TU BACK	OSL	OUTSTANDING LEG
BF	BACK FACE	OPNG	OPENING
BOI		OPP	OPPOSITE
BRG	BEARING	PC	PIECE
BS	BUTH SIDES	PL	PLATE
BTWN	BETWEEN	PP	PANEL POINT
C TO C / CC	CENTER TO CENTER	PPGW	PARTIAL PENETRATION
CANT	CANTILEVER		GROOVE WELD
CHD	CHORD	PTFE	POLYTETRAFLUOROETHYLENE
CI	CAST IRON	EXTER	EXTERIOR
CL	CENTER LINE	INTER	INTERIOR
CLR	CLEAR	LG	LENGTH
COL	COLUMN	PVC	
CONC	CONCRETE	PVMT	
CONN	CONNECTION		
CONSTR	CONSTRUCTION		
CONT	CONTINUOUS	PDWY	
CORR	CORRUGATED		
CPGW	COMPLETE PENETRATION		
CTSK		REINF	
		REQU	
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HEY		IYP	IYPICAL
		UON / UN	UNLESS OTHERWISE NOTED
		VERT	VERTICAL
IT		WP	WORKING POINT
		W/	WITH
	MUUIFILU		
	OF TRAFFIC		
NA	NOT APPLICABLE		
NIC	NOT IN CONTRACT		

				DRAWN BY	P. MANAYATH	Therater Temproti	AND	Triborough	DRAWING TITLE		CONTRACT	NO. GFM-520N
				DESIGNED BY	L. HERBERT	40 WALL STREET		Fridge and Tunnel				C 2
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005	Authority		AND ABBREVIATIONS			G−∠ SHEET 2 OF 12
"IT IS A	'IT IS A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS DRAWING IN INT WAY, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER/ARCHITECT AS							BARRIER CRASH TESTING			DATE	JULY 6, 2018
APPL 1CAE FOLLOWED	BLE. THE ALTERING ENGINEER/ARCHITECT SHALL AFFIX HIS/HER SEAL AND THE N D BY HIS/HER SIGNATURE AND DATE OF ALTERATION."	DTATION 'A	LTERED BY'	SCALE: NC	ONE			FOR PROJECT TN-49	PROJECT NO.	GFM-520N, WORK ORDER #14	REVISIO	ND. 0
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ABBREVIATIONS

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GENERAL:

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- 1. THE SCOPE OF WORK IN THE PROJECT CONSISTS OF THE FOLLOWING
 - A. FABRICATION OF DECK PANEL AND 4 RAIL STEEL BARRIER.
 - B. OBTAIN SERVICES OF A SHOP INSPECTION AGENCY, ACCEPTED BY THE ENGINEER, TO WITNESS, INSPECT, AND CERTIFY THAT ALL STRUCTURAL STEEL FURNISHED HAS BEEN FABRICATED IN ACCORDANCE WITH REQUIREMENTS STIPULATED BY CONTRACT DOCUMENT AND SPECIFICATIONS.
 - C. DELIVERY OF ORTHOTROPIC DECK AND SUPPORTS, 4 RAIL STEEL BARRIER, BOLTS, ANCHORS, AND PLATES TO TESTING FACILITY.
- 2. IF THE CONTRACTOR DAMAGES ANY EQUIPMENT OR MATERIALS WHICH ARE TO REMAIN IN PLACE OR ARE THE PROPERTY OF THE TESTING FACILITY, DAMAGED MATERIAL SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE TESTING FACILITY AT THE EXPENSE OF THE CONTRACTOR.
- 3. THE CONTRACTOR SHALL PROMPTLY REMOVE ALL EXCESS MATERIALS. CONSUMERABLES AND TOOLS FROM THE TESTING FACILITY AFTER COMPLETION OF THE WORK.
- 4. THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE TESTING FACILITY FOR ACCESS, POSSIBLE OBSTRUCTIONS, AND STAGING AREAS BEFORE FABRICATION OF DECK PANELS AND SUPPORTING FRAMES.
- 5. DESIGN SPECIFICATIONS:
 - A. NYSDOT LRFD BRIDGE DESIGN SPECIFICATIONS WITH ALL PROVISIONS IN EFFECT AS OF OCTOBER 2014.
 - B. NY STATE STEEL CONSTRUCTION MANUAL (NYSSCM) AND ADDENDUMS OCTOBER 2013.
 - C. NYSDOT BRIDGE DESIGN MANUAL 4TH EDITION WITH CURRENT ADDENDUM.
 - D. AASHTO/AWS D1.5-2015 BRIDGE WELDING CODE.
 - E. AASHTO/17TH EDITION.

STRUCTURAL STEEL:

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- 1. MATERIALS SHALL CONFORM WITH THE FOLLOWING, UNLESS OTHERWISE NOTED
 - A. ORTHOTROPIC DECK STEEL ASTM A709, GRADE 50T, ZONE 2 CVN TOUGHNESS
 - B. STRUCTURAL TUBES STEEL ASTM A500, GRADE 46B.
 - C. BARRIER TUBES STEEL ASTM A500, GRADE 46B, GALVANIZED
 - D. BARRIER POSTS STEEL ASTM A709, GRADE 50, GALVANIZED
 - E. ALL OTHER STRUCTURAL STEEL STEEL ASTM A709, GRADE 50
 - F. H.S. BOLTS ASTM F3125 GR A325. TYPE 1. GALVANIZED
 - G. NUTS ASTM A563: WASHERS ASTM F436, GALVANIZED
- 2. ALL FASTENERS SHALL BE 7/8" DIAMETER AND SHALL INCLUDE HEAVY HEXAGONAL NUTS AND HARDENED WASHER UNDER THE PART TURNED, UNLESS OTHERWISE NOTED. ALL FASTENERS SHALL NOT BE REUSED.
- 3. ALL CONNECTIONS SHALL BE SLIP-CRITICAL UNLESS OTHERWISE NOTED.
- 4. UNLESS OTHERWISE NOTED, ALL ANCHOR BOLTS SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHORS OR APPROVED EQUAL.
- 5. EXCEPT AS REQUIRED ON THE PLANS AND IN THE SPECIFICATIONS. WELDING SHALL CONFORM TO THE NYSDOT STEEL CONSTRUCTION MANUAL, LATEST EDITION INCLUDING CURRENT ADDENDA AND SPECIFICATIONS. WELDING SHALL ONLY BE ON STEEL MEMBERS PROVIDED BY THE CONTRACTOR. WELD TO STEEL PROVIDED BY THE TESTING FACILITY SHALL NOT BE ALLOWED UNLESS APPROVED BY THE ENGINEER.
- 6. FILLET WELD SIZES NOT INDICATED SHALL BE 5/16" MINIMUM.
- 7. ALL WELDS SHALL BE MADE WITH MATCHING FILLER MATERIAL. PROVIDING ZONE 2 CVN TOUGHNESS IN THE "AS WELDED CONDITION".
- 8. ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.5 UNLESS NOTED OTHERWISE. ALL WELDS SHALL DEVELOP THE FULL STRENGTH OF THE MEMBERS OR COMPONENTS BEING CONNECTED UNLESS NOTED OTHERWISE.
- 9. ALL WELDS SHALL BE 100% VISUALLY INSPECTED. FILLET WELDS SHALL RECEIVE 20% MT.

				DRAWN BY	P. MANAYATH	Thornton Tomasetti	en e	Friborough	RAWING TITLE		CONTRACT NO. GFM-520N	
				DESIGNED BY	L. HERBERT	40 WALL STREET		Bridge and Tunnel			NO. G_3	
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005	00000000000000000000000000000000000000	the Authority		GENERAL NOTES		
"IT IS A ANY WAY,	"IT IS A VIQLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS DRAWING IN ANY MAY, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER/ARCHITECT AS APPLICABLE. THE ALTERING DENOINEER/ARCHITECT SHALL AFFIX HIS/HER SEAL AND THE NOTATION 'ALTERED BY' FOLLOWED BY HIS/HER SIGNATURE AND DATE OF ALTERATION."							BARRIER CRASH TESTING			DATE JULY 6, 2018	
APPLICAB FOLLOWED				SCALE: NO	IONE	FOR PROJECT TN-49		ROJECT NO.	GFM-520N, WORK ORDER #14	REVISION NO.		
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- FABRICATOR.

2. ALL STEEL PIECES SHALL BE KEPT COVERED, CLEAN, AND DRY WHILE STORED.

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HANDLING AND STORAGE MATERIALS:

1. THE TESTING FACILITY AND CONTRACTOR ARE RESPONSIBLE FOR THE HANDLING AND STORAGE OF ALL CONSTRUCTION MATERIALS. INCLUDING ALL STEEL PIECES SUPPLIED BY THE STEEL

					P. MANAYATH	Thornton T	ſomasetti	Triborough	
				DESIGNED BY	L. HERBERT	40 WALL S	STREET	Bridge and	Tunnel
EV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YOHK,	, NY 10005	Authority	
T IS A	A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS	DRAWING I	N						BARRIER CI
PLICAE	SLE. THE ALTERING ENGINEER/ARCHITECT SHALL AFFIX HIS/HER SEAL AND THE N D BY HIS/HER SIGNATURE AND DATE OF ALTERATION."	IOTATION 'A	LTERED BY'	SCALE: A	S NOTED				FOR PRC

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REVISION NO. 0

GFM-520N, WORK ORDER #14

				DESIGNED BY	L. HERBERT	40 WALL STREET	Bridge and	Tunnel
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005	Authority	
"IT IS ANY WAY APPLICA FOLLOWE	A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THI- .UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER. BLE. THE ALTENING ENGINEERARCHITECT SHALL AFTIX HIS/HER SEAL AND THE N D BY HIS/HER SIGNATURE AND DATE OF ALTERATION."	S DRAWING I /ARCHITECT NOTATION 'A	N AS LTERED BY'	SCALE: A	S NOTED			BARRIER CRASH TESTING FOR PROJECT TN-49

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DATE JULY 6, 2018 REVISION NO. 0 GFM-520N, WORK ORDER #14 4

DIAPHRAGM DETAILS - 2

9" 6" 3" 0

3"

SCALE: 11/2"=1'-0"

SCALE: 3"=1'-0"

DRAWING

6''

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CONTRACT NO. GFM-520N

S–6

SHEET 9 OF 12

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TABLE 1	- POST INSTALLATION							
POST NUMBER	POST TYPE	"A"						
1L	NEW	4'-11' ₄ "						
2L	EXISTING	9'-3 ³ /4"						
3L	NEW	14′-9 ³ ⁄4″						
4L	NEW	19'-9"						
5L	NEW	24'-8'/4"						
6L	NEW	29'-7' ₂ "						
7L	NEW	34'-6 ³ /4"						
8L	NEW	39'-6"						
9L	NEW	44'-5'/4"						
10L	NEW	49'-4' ₂ "						
11L	NEW	54'-3 ³ /4"						
12L	EXISTING	58'-9 ³ /4"						
13L	NEW	64'-2'/4"						
14L	NEW	69'-1' ₂ "						
1R	NEW	4'-11'/4"						
2R	EXISTING	9'-3 ³ /4"						
3R	NEW	14'-9 ³ /4"						
4R	NEW	19'-9"						

TABLE 2	TABLE 2 – REPLACEMENT PARTS											
PIECE	QUANTITY	LENGTH										
HSS 5x3x ¹ /2"	2	40'-0"										
HSS 6x6x ³ /8"	2	40'-0"										
W8×28 POST	3	AS DETAILED										
1 ³ /4" BASE PLATE	3	AS DETAILED										
L5×5× ⁵ ⁄8″	3	6 ¹ /2"										
1 " BOLT	12	4 ¹ /2"										
1 " BOLT	12	1'-7'/2"										
7∕8″BOLT	12	7″										
7∕8″ BOLT	6	8″										
³ /4" BOLT	3	8″										
³ ⁄ ₄ " BOL T	6	21/2"										

TRIBOROUGH BRIDGE AND TUNNEL AUTHORITY

PROJECT GFM-520N, WORK ORDER #14 TEST PIT DETAILS FOR PROJECT TN-49

JULY 6, 2018

PREPARED BY: Thornton Tomasetti 40 WALL STREET, NEW YORK, NY 10005

7-26-18

DATE

SAMUEL SUMMERVILLE, P.E. PRINCIPAL - THORNTON TOMASETTI

1		2			3	
DRAW	ING LIST		SYMBOLS			
SHEET DWG. REV. D	RAWING TITLE		 - NEW HIGH STRENG	TH BOLT.	ABT	ABOUT

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HEET NO.	DWG. NO.	REV. NO.	DRAWING TITLE	- + -	NEW HIGH STRENGTH BOLT.	ABT ANCH	ABOUT ANCHORAGE
			CENEDAL			AOBE	AS ORDERED BY ENGINEER
1	C 1		CENERAL			ASTM	AMERICAN SOCIETY OF
ו ר	6-1	0	TITLE SHEET AND LUCATION PLAN		NEW SLOTTED HOLE.		LESTING MATERIALS
2	6-2	0	GENERAL NUTES, DRAWING LIST, STMBOLS AND ADDREVIATIONS			B 10 B / BB	BACK FACE
						BOT	
			STRUCTURAL	٩٩		BBG	
٦	P_1	0		0 0	CONCRETE.	BS	BOTH SIDES
1	P_2	0	EDITING AND RETAINING WALLS DETAILS - 1			B T WN	
5	P_3	0	EQUING AND RETAINING WALLS DETAILS - 2				CENTER TO CENTER
5	P_4	0	EQUING AND RETAINING WALLS DETAILS - 3				CANTLEVER
7	P_5	0	EQUING AND RETAINING WALLS DETAILS - 4	LEGENDS FOR SE	ECTION MARKS	CHD	
, 0	P_6	0	POULING AND RELAINING WALLS DETAILS - 4			CI	
0	P_7	0	REINFORCEMENT DETAILS - 1 DE INFORCEMENT DETAILS - 2		DETAIL MARK	CI	
9 10		0	REINFURCEMENT DETAILS - 2				CENTER LINE
10	F-0	0	STEEL RAILING FUST ANCHURAGE DETAILS		$\frac{1}{1}$	CLR	CLEAR
				view e		COL	COLUMN
					DRAWING NUMBER	CONC	CONCRETE
	- <i>.</i>				ON WHICH DETAIL	CONN	CONNECTION
11	D-1	0	TEST PIT DEMOLITION PLAN		ISTAKEN	CONSTR	CONSTRUCTION
12	D-2	0	DEMOLITION DETAILS - 1		- SECTION MARK	CONT	CONTINUOUS
13	D-3	0	DEMOLITION DETAILS - 2			CORR	CORRUGATED
14 15	D-4 D-5	0 0	DEMOLITION DETAILS – 3 DEMOLITION DETAILS – 4	SECTION SCALE: 00	<u>1</u> <u>0</u> -00	CPGW	COMPLETE PENETRATION GROOVE WELD
					4		
						DET	
					IS TAKEN	DIA	
NOTE	S:				- DETALL MARK		
GENER	AL:					DWC	
1. W(ORK THESE	DRAWINGS	WITH "BARRIER CRASH TESTING FOR	DETAIL A		ED	
FI	RUJECI IN	-49.	1	00-	-00	EF	
2. TH	HE SCOPE	OF WORK F	OR THIS PROJECT CONSISTS OF	i			
DE	ECK PANEL	AND 4 RA	IL STEEL BARRIERS.	/	ON WHICH DETAIL		
,					IS SHOWN		
5. II Wi	F THE CUN HICH ARE	TO REMAIN	IN PLACE OR ARE THE PROPERTY OF THE	-	◀	EXIST / EXST	
TE	ESTING FA	CILITY, D	AMAGED MATERIALS SHALL BE REPAIRED OR			EXP	
A.	EPLACED I T THE EXP	N A MANNE ENSE OF T	HE CONTRACTOR.		- SECTION MARK	F	FARENHEIT
				4	1/	FF	FRUNT FACE
4. TH M/	HE CONTRA ATERIALS.	CTOR SHAL CONSUMER	L PROMPTLY REMOVE ALL EXCESS MABLES, AND TOOLS FROM THE TESTING	V	0-00		
F	ACILITY A	FTER COMP	PLETION OF THE WORK.		DRAWING NUMBER	FLBM	
5. TI			I PROVIDE WORK PLATEORMS AND SUPPORTS		UN WHICH SECTION IS SHOWN	FLG	FLANGE
A:	S REQUIRE	D FOR THE	ERECTION OF THE SUPPORT TRUSSES AND			FUT	
DE	ECK PANEL	• THE COS	T OF ALL PLATFORMS AND SUPPORTS SHALL				
DE	ESIGN WOR	K SHALL B	E SIGNED AND SEALED BY A PROFESSIONAL				
13 S	NGINEER C		LICENSED IN THE STATE OF TEXAS AND				
SU	UDMITED	IU INE EN	WINEER FUR REVIEW AND AFFRUVAL.				HICH STRENCTH
STRUC	TURAL STE	EL:				HS	HIGH SIRENGIH
I AL Pf	LL SIRUCI ROVIDED B	URAL SIEE Y THE STE	L. BULIS, ANUHURS, AND PLAIES SHALL BE			JI	
						MAX	
2. UI Pi	NLESS OTH FMOVED. S	ERWISE NO	TION DRAWINGS FOR DETAILS.			MIN	
Λt			TON UNREINUS FOR DETRIES.			MOD	MODIFIED

				DRAWN BY	P. MANAYATH	Thornton Tomasetti	States and the	Triborouah	DRAWING TITLE		CONTRAC	!
				DESIGNED BY	L. HERBERT	40 WALL STREET		Bridge and Tunnel	GENEF	AL NOES, DRAWING LIST, SYMBOLS	DRAWING NO.	i
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005	0. 	Authority		AND ABBREVIATIONS		
"IT IS A ANY WAY,	VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER/	DRAWING	IN AS					TEST PIT DETAILS			DATE	J
PPLICABL	E. THE ALTERING ENGINEER/ARCHITECT SHALL AFFIX HIS/HER SEAL AND THE N By HIS/HER SIGNATURE AND DATE OF ALTERATION."	DTATION '	ALTERED BY'	SCALE: NO	ONE			FOR PROJECT IN-49	PROJECT NO.	GFM-520N, WORK ORDER #14	REVISIO	JN
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-- NUMBER

-- NOMINAL -- NEAR SIDE

-- NOT TO SCALE

-- ON CENTER -- OUTSTANDING LEG

-- OPENING

-- OPPOSITE

-- EXTERIOR

-- INTERIOR

-- PAVEMENT

-- LENGTH

-- PARTIAL PENETRATION GROOVE WELD

-- POLYVINYLCHLORIDE

-- POLYTETRAFLUOROETHYLENE

-- PIECE

-- PLATE -- PANEL POINT D

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R / RAD

STIFF STL STR / STRG SYMM TBTA

THD THRD тнк THRU TΡ ΤYΡ

UON / UN

VERT

WP W/

-- RADIUS -- REINFORCED CONCRETE -- ROADWAY -- REFERENCE -- REINFORCEMENT -- REQUIRED -- RELIEF JOINT -- SIDEWALK

-- SECTION -- SPACE / SPACING -- SPLICE -- SQUARE -- STAINLESS STEEL

-- STIFFENER -- STEEL -- STRINGER

-- SYMMETRICAL -- TRIBOROUGH BRIDGE AND TUNNEL AUTHORITY

-- THREAD

-- THREADED

-- THICK -- THROUGH

-- TURNING POINT

-- TYPICAL

-- UNLESS OTHERWISE NOTED

-- VERTICAL

-- WORKING POINT

-- WITH

-- MAINTENANCE AND PROTECTION OF TRAFFIC

-- NOT APPLICABLE

MPT NA

NIC

-- NOT IN CONTRACT

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				DRAWN BY	P. MANAYATH	Thornton Tomasetti	Triborough
				DESIGNED BY	L. HERBERT	40 WALL STREET	Bridge and Tunnel
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005	Some state of the second secon
"IT IS A ANY WAY	A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THI UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER	S DRAWING I	N AS				TEST PIT DETAILS
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OR CLARITY.	SEE	DRAWINGS
ETAILS.		

	sc	ALE: 1"•1'-0"
TITLE		CONTRACT NO. GFM-520N
OTING	AND RETAINING WALL DETAILS – 2	DRAWING NO. P-3 SHEET 5 OF 15
		DATE JULY 6, 2018
N0.	GFM-520N, WORK ORDER #14	REVISION NO. O
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TABLE 1	– POST INSTA	LLATION
POST NUMBER	POST TYPE	"A"
1L	1	4'-11'/4"
2L	2	9'-3 ³ /4"
3L	3	14′-9 ³ ⁄4″
4L	3	19'-9"
5L	3	24'-8'/4"
6L	3	29'-7' <u>⁄2</u> "
7L	3	34'-6 ³ /4"
8L	3	39′-6″
9L	3	44' -5'/4"
10L	3	49'-4'/2"
11L	3	54'-3 ³ /4"
12L	2	58'-9 ³ /4"
13L	3	64'-2'/4"
1 4L	1	69'-1' ₂ "
1 R	1	4'-11' ₄ "
2R	2	9'-3 ³ /4"
3R	3	14′-9 ³ ⁄4″
4R	1	19'-9"

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										1' 6'' 	0 1' 2' CALE: 1''-1'-0''
				DRAWN BY	P. MANAYATH	Thornton Tomasetti	of Set The	Triborouah	DRAWING TIT	LE	CONTRACT NO. GFM-520N
				DESIGNED BY	L. HERBERT	40 WALL STREET		Bridge and Tunnel			
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	C. CLARK	NEW YORK, NY 10005		Authority	STEEL RAILING POST ANCHORAGE DETAILS		SHEET 10 DE 15
"IT IS A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS DRAWING IN								TEST PIT DETAILS			DATE JULY 6, 2018
APPL ICAE FOLLOWED	BLE. THE ALTERING CHIEFENANCHIEFENANCHIEFENANCHIEFEN BLE. THE ALTERING CHIEFENANCHIEFEN SHALL ART SHALL AND THE BY HIS/HER SIGNATURE AND DATE OF ALTERATION."	NOTATION '	ALTERED BY'	SCALE:	NOT TO SCALE			FOR PROJECT TN-49	PROJECT NO.	GFM-520N, WORK ORDER #14	REVISION NO. O
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NOTES:

- CONCRETE SHALL BE IN ACCORDANCE WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF F'C=4000 psi.
- REINFORCING STEEL SHALL BE EPOXY COATED AND CONFORM TO ASTM A615, GRADE 60.
- MINIMUM CLEAR COVER TO REINFORCING STEEL SHALL BE 2", UNLESS NOTED OTHERWISE.

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